

Behaviour of Anchored Pile Wall Under Dynamic Load & Earthquake

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ABSTRACT:- The purpose of this study is to observe the behaviour of concrete pile wall that to be constructed in the Gazino Station of Ulus–Keçiören under the determined soil properties and site conditions, the studied area consisting existing four-storey building, excavation of two layers and supporting the pile wall with anchors, the water pressure is involved in this study and the ground water level is changeable to three levels, dynamic load to be applied and concentrated on the top of pile wall which will be subjected to earthquake on basement recorded by USGS in 1989.

Keywords:- Plaxis, Pile wall, Material behaviour, water pressure, Dynamic load, earthquake.

I. INTRODUCTION

The problem modelled with a geometry model of 35 m width and 20 m depth, A ground anchor modelled by a combination of a node-to-node anchor and a geotextile, The geotextile simulates the grout body whereas the node-to-node anchor simulates the anchor rod. In reality there is a complex three dimensional state of stress around the grout body. Although the precise stress state and interaction with the soil cannot be modelled with this 2D model, it is possible in this way to estimate the stress distribution, the deformations and the stability of the structure on a global level, assuming that the grout body does not slip relative to the soil. With this model it is certainly not possible to evaluate the pull-out force of the ground anchor.

The pile wall is modelled as a beam. The interfaces around the beam are used to model soil-structure interaction effects. They are extended under the wall for 1,0 m. Interfaces should not be used around the geotextiles that represent the grout body.

The water table is below the excavation level but initially there is a water level to be raised steeply and water pressures will effect of pile wall and existing building. As the excavation commences water drained and no groundwater is observed during excavation stages. The schematic view of the analysed model of the studied section of the Gazino station is given in Fig. 1

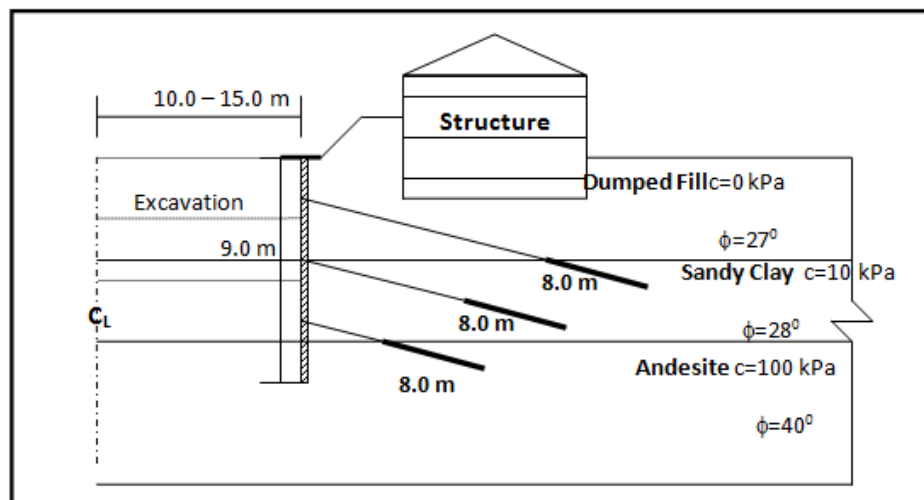


Fig.1 Modelled section

II. MATERIALS AND METHODS

2.1 Soil and interface properties:-

The properties of soil three layers and interface materials are assumed and obtained from the plaxis tutorial manual-V8 as the following table :

Table 1 Soil and interface properties

Parameter	Name	Fill	Sandy Clay	Andesite	Building Fill	Unit
Material model	<i>Model</i>	MC	MC	MC	MC	
Type of material behaviour	<i>Type</i>	drained	drained	drained	drained	-
Dry soil weight	γ_{dry}	17,5	20	24	17	kN/m ³
Wet soil weight	γ_{wet}	19	23	27	20	kN/m ³
Horizontal permeability	Kx	0	0	0	5	m/day
Vertical permeability	Ky	0	0	0	5	m/day
Young's modulus	E_{ref}	4000	20000	200000	30000	kN/m ²
Poisson's ratio	ν	0,353	0,346	0,263	0.2	-
Cohesion	C_{ref}	0,2	10	100	1.0	kN/m ²
Friction angle	ϕ	27	28	40	35	°
Dilatancy angle	ψ	0	0	10	4	°
Interface reduction factor	<i>Rinter</i>	Rigid	Rigid	Rigid	Rigid	-
Interf Permeability	<i>Perm.</i>					-

2.2 Properties of the plates:-

The properties of plates members including:

- Pile wall
- Pile cap
- Building beams

All these plates are assumed and obtained from the plaxis tutorial manual-V8 as the following tables

Table 2 Pile (beam) Properties

Parameter	Name	Value	Unit
Type of behaviour	<i>Material type</i>	Elastoplastic	-
Normal stiffness	EA	$2.4 \cdot 10^7$	kN/m
Flexural rigidity	EI	$1.28 \cdot 10^6$	kNm/m
Equivalent thickness	D	0.80	m
Weight	W	19.2	kN/m/
Poisson's ratio	ν	0.15	-

Table 3 Pile cap (beam) Properties

Parameter	Name	Value	Unit
Type of behaviour	<i>Material type</i>	Elastoplastic	-
Normal stiffness	EA	$1.5 \cdot 10^7$	kN/m
Flexural rigidity	EI	$3.125 \cdot 10^5$	kNm/m
Equivalent thickness	D	0.50	m
Weight	W	12	kN/m/
Poisson's ratio	ν	0.15	-

Table 4 Building members Properties

Parameter	Name	Value	Unit
Type of behaviour	<i>Material type</i>	Elastoplastic	-
Normal stiffness	EA	$5 \cdot 10^5$	kN/m
Flexural rigidity	EI	$9 \cdot 10^5$	kNm/m
Equivalent thickness	D	0.147	m
Weight	W	5	kN/m/
Poisson's ratio	ν	0	-

3.2 Pile wall behavior under dynamic load:-

The displacement is increasing with time progress specially at phase 3 after applying the dynamic load, the most effected area is around the anchor area and in the area that located between the building and pile wall as it viewed from plastic points figure3.

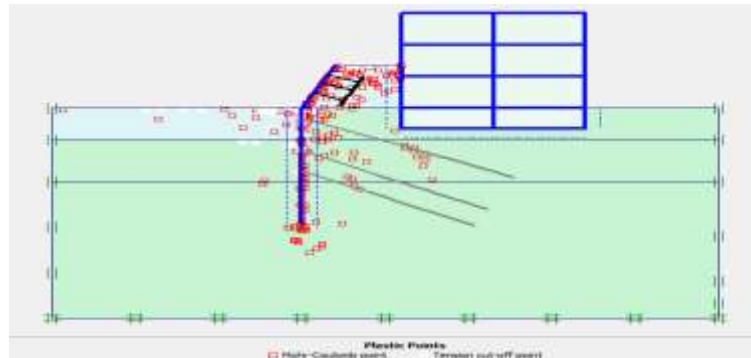


Fig.3 shows the plastic points

3.3 Pile wall behavior under Earthquake:-

The maximum vertical displacement occurred in the bottom of basement due to applying of earthquake load, The displacement of top and bottom of pile wall observed with same values (approximately), but suddenly at dynamic time of 4.08 s big different in the displacement between top and bottom pile wall has been observed, the top pile wall displacement values are bigger than the displacement values of the bottom pile wall, which is clear vesible in the figure4.

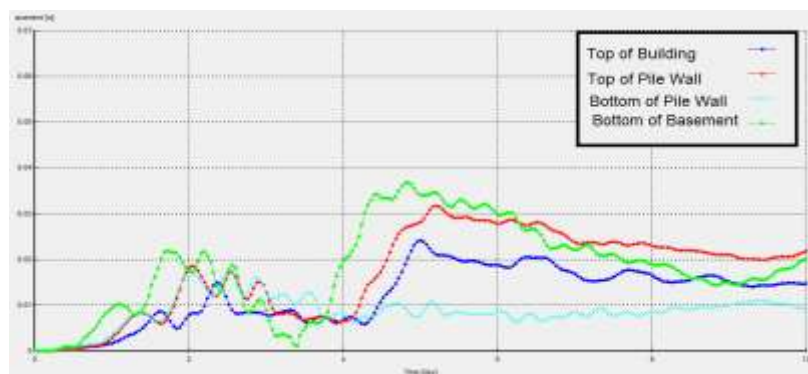


Fig.4 The pile wall time Vs Displacement

IV. CONCLUSION

The results shows that pile wall affected negatively with the earthquake and the dynamic load cases while it is not effected in case of raising the ground water level, and we recommended to change and improve the properties of materials and plates for guarantying stable pile wall and also keeping the existing building not affected.

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