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Retention Pond Analysis as Flood Control in Kampung Sawah Area, Martapura District, East OKU Regency

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Abstract
This study was conducted to obtain a retention pond design as a flood controller in the Kampung Sawah water catchment area of East OKU Regency.
The data used is rainfall data for 11 years as secondary data and the rainfall frequency is analyzed to obtain the IDF curve. then the flow rate with a certain return period is obtained.
The results obtained from this study are the amount of the planned discharge for the retention pond is 14.403 m^3/sec so that it is very ideal for the volume of the retention pond as a flood controller in the Kampung Sawah area, Martapura district, East OKU Regency and the amount of the retention pond volume (ΔQ) = 37.923 m^3 with the dimensions of the pond planned next to the channel with a pond height (h) of 1.00 meters, so for the retention pond a minimum area of 37.92 m^2 is needed.
***Keywords**: Rainfall data, IDF-Curve, Discharge, Retention-pond*

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I. INTRODUCTION

OKU TIMUR Regency is geographically located at 103o 40'– 104o 33' East Longitude and 3o 45'– 4o 55' South Latitude. In accordance with Law Number 37 of 2003, the area of Ogan Komering Ulu Timur Regency (OKU TIMUR) is 3,370 km², where most of the area is lowland and tends to be flat except in the Martapura District and its surroundings which tend to be hilly. (OKUT in figures, 2022) Topographically, the OKU TIMUR Regency area can be classified into flat areas (Peneplain Zone), undulating (Piedmont Zone), and some are hilly areas that have varying elevations, namely between 42 meters to the highest elevation reaching 87 meters above sea level (asl) and slope gradients varying between 0-2% and 2-15%. Flat areas are found in Belitang District and Buay Madang District, while hilly areas are found in part of Martapura District. (OKUT in figures, 2022)

The physiography of OKU TIMUR district is part of the Barisan Mountains Zone and the Basin Zone. The Barisan Mountains Zone is characterized by a landscape of volcanic cones, mountains and undulating hills formed by intrusive rocks of andesitic-granite composition, pyroclastic and Tertiary sedimentary rocks; while the Basin Zone is characterized by a landscape of low and gentle undulating plains which are mostly formed by river alluvial deposits; in some places there are Tertiary sedimentary rocks and local swamp deposits and reef limestone. (Satria Jaya Priatna et al., 2011)

In OKUT district, the Komering River flows in the southern part of Sumatra Island which has a tropical rainforest climate. The average temperature per year is around 24 °C. The hottest month is October, with an average temperature of 26 °C, and the coldest is January, around 22 °C. The average annual rainfall is 2902 mm. The month with the highest rainfall is November, with an average of 435 mm, and the lowest is August, with an average of 83 mm. The Komering Sub-DAS is one of the nine Musi Sub-DAS and is located in the southern part of the island of Sumatra, which has an area of 915,375.820 ha. (OKUT in figures, 2022)

One of the serious problems that hit several urban and rural areas in Indonesia is flooding. The flood is not purely due to natural factors alone, but rather due to uncontrolled changes in land use without considering the sustainability of the river basin from upstream to downstream. The part that must be considered in flood control is not only surface flow, but also runoff. The rate and volume of runoff are influenced by the distribution and intensity of rainfall throughout the watershed. (Achmad Syarifudin, 2018) In hydrology, rainfall is an important input component where the analysis of rainfall data in the review of hydrological planning aspects is used as an approach in estimating the amount of flood discharge that occurs in a watershed. The approach to estimating flood discharge that occurs from rainfall data is carried out if the watershed in question is not equipped with an Automatic Water Level Recorder (AWLR) water measuring instrument. To obtain the amount of rainfall that can be considered as the actual depth of rainfall that occurs throughout the watershed, a number of rainfall stations are needed that can represent the amount of rainfall in the watershed. (Achmad Syarifudin, 2017) In addition to rainfall data, surface runoff is an important factor in the transport system of various materials that will be carried into river flow. If the intensity of rainfall exceeds the infiltration rate, excess water begins to accumulate as surface reserves. If the surface reserve capacity is exceeded, surface runoff begins as a thin layer flow. Surface runoff is the part of the runoff that passes over the land surface towards the river channel (Seyhan 1990).

In high intensity rain, the total volume of surface runoff will be greater than in low intensity even though the total rainfall received is the same. Topographic forms such as land slope will affect surface runoff. Watersheds with high slopes will produce greater surface runoff. The presence of vegetation can increase the amount of water retained on the surface, thereby reducing the rate of surface runoff. (Achmad Syarifudin, 2018).

To create a realistic model is needed into creating retention ponds to control flooding in urban areas of the natural drainage and their related threats a methodology is generated in this paper.

II. MATERIAL AND METHODS

This research took place in the river/channel of the Sawah Village, Martapura District in the East OKU Regency area. (Figure 1)

Secondary data, namely by collecting rainfall data to conduct frequency analysis and cross-sections and lengths taken from previous research conducted by Lanosin (2023).

Primary data, namely by conducting a hydrodynamic flow survey to determine the flow speed in the river/channel of the Sawah Village area in the Water Catchment Area (KTA) as in the map in Figure 2.

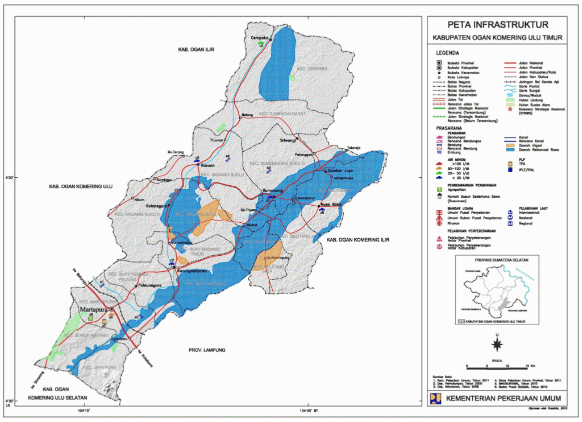

Figure 1: Infrastructure map of East OKU Regency

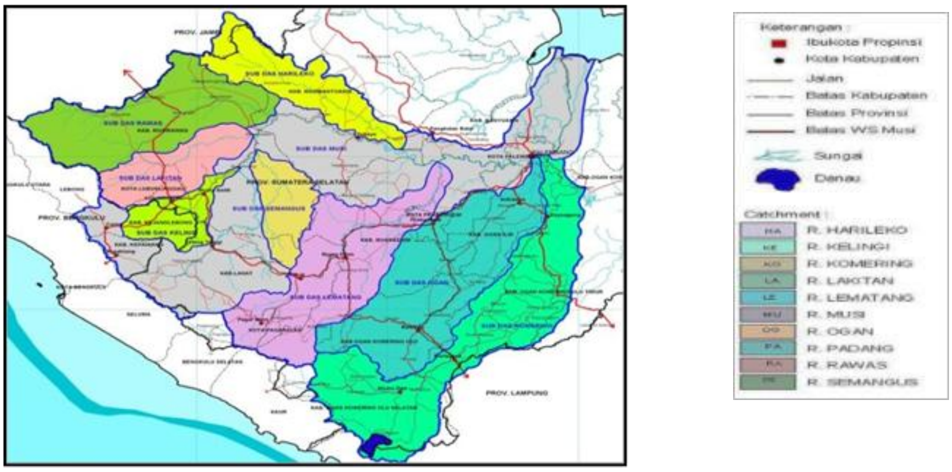

Figure 2: Komering River Basin

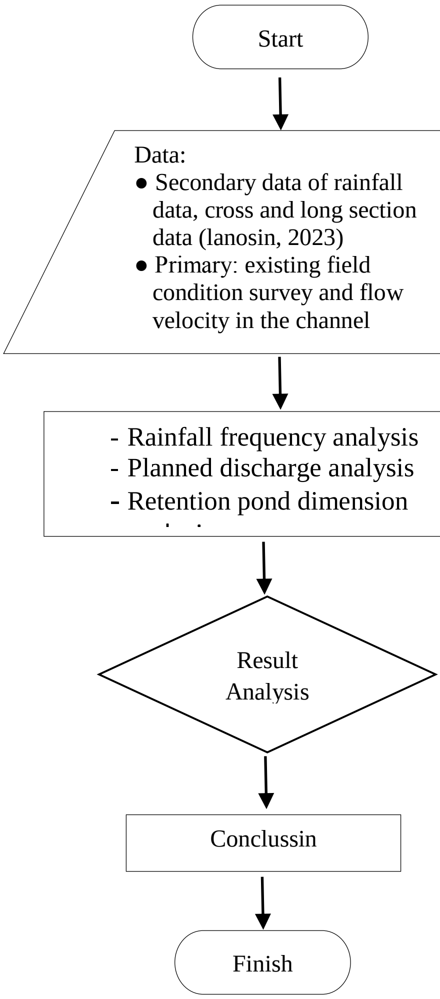

Figure 3: The flow of the methodology

III. RESULTS AND DISCUSSION

3.1. Selection of Return Period

Determination of return period based on the method:

1. Empirical Method

Past event observation data to predict future events with the same magnitude. The probability of extreme events in "N" years will recur in the next "n" years is expressed as:

P (N,n) = n / N + n .... (1)

2. Risk Analysis

The risk of failure of the planned building is a risk analysis expressed in the equation:

R = 1 - 1 - 1/T n .... (2)

with:

R = Probability where Q Qt occurs at least once in n years.

3.2. Hydrology

Considering the availability of hydrometric data is not yet available properly, rainfall data is used as the basis for hydrological calculations. The rainfall data used is rainfall data recorded by several stations in the planning area and has quite long data, namely from 2011 to 2021 and the average rainfall value is taken from the maximum monthly rainfall data.

For the planned rainfall estimate, frequency analysis is used by reviewing the commonly used distribution:

1. Planned Rain Estimate

a. For Return Periods above 1 year

The planned rainfall estimate is carried out by analyzing the frequency of the annual maximum rainfall data (annual series). There are several distributions in statistics and those commonly used in frequency analysis are 4 (four) types, namely:

1). Normal
2). Gumbel type I
3). Log normal 2 parameters
4). Pearson type III log

Each distribution has its own statistical properties. By calculating the statistical parameters of the analyzed data series, it can be estimated which distribution is appropriate for the data series. The statistical parameters in question are as follows:

$$X = \frac{\Sigma xi}{n} \qquad \text{...} \qquad (3)$$

$$S = \sqrt{\frac{(xi - xr)^2}{(n-1)}} \qquad \text{...} \qquad (4)$$

$$Cs = \frac{n}{(n-1)(n-2)S^3} \sum (xi - xr)3 \qquad \text{...} \qquad (5)$$

$$Ck = \frac{n}{(n-1)(n-2)(n-3)S^4} \sum (xi - xr)4 \qquad \text{...} \qquad (6)$$

with:
xr = Mean value
S = Standard deviation
Cs = Skewness coefficient
Ck = Curtosis coefficient
xi = rainfall data
n = amount of data
The typical statistical properties of each distribution can be explained as follows:
1). Normal Distribution: Cs = O
Typical characteristics: Cs = 0
Probability P (x-S) = 15.87%
P (x) = 50.00%
P(x+s) = 84.14%

The possibility of a variable in the interval x – S and X + S = 68.27% and in the interval X – 2S and X + 2S = 95.44%.

2). Log normal distribution (2 parameters)
Characteristics: Cs = 3 Cv
Cs is always positive
Probability line equation: x(t) = x + K
With x(t) = rainfall depth with recurrence period t (years) K = Frequency factor
3). Gumbel distribution type I
Characteristics: Cs = 1.3960 cv and Ck = 5.4002

Probability line equation:

$$X(t) = x + \frac{\sigma}{\sigma n}(y - yn) \qquad \text{...} \qquad (7)$$

with: Y = reduced variated
yn and n = Mean value and standard deviation of reduced variated.

4). Pearson Log Distribution type III
The statistical data does not approach the characteristics of the three previous distributions.
The rainfall data is transformed into its natural logarithm value so that the xi values change to ln xi. Then the average value, standard deviation and skewness coefficient are calculated as follows:

$$\overline{ln \frac{\sum_{i=1}^{n} ln\, xi}{n}} \qquad \text{...} \qquad (8)$$

$$S = \sqrt{\frac{\sum_{i=1}^{n}(ln\, xi - \overline{ln}\quad x)^2}{(n-1)}} \qquad \text{...} \qquad (9)$$

$$Cs = \frac{n}{(n-1)(n-2)S^3}\sum_{i=1}^{n}(\ln xi - \overline{\ln}\, x)^3 \qquad \text{...} \qquad (10)$$

Probability line equation:

$$\ln x(t) = \overline{\ln}\, x + K\, S \qquad \text{...} \qquad (11)$$

K is the frequency factor. based on the Cs value calculated in equation 11, the depth of rainfall with a return period of t years is obtained by finding the antilogarithm of the ln (t) value.

To find out whether the existing data is in accordance with the selected theoretical distribution, a goodness of fit test is carried out using the Smirnov Kolmogorov and chi-square tests.

a. For Return Period Less than 1 year

The estimated planned rainfall with a return period of less than 1 year cannot be done using the frequency analysis above. Determining the depth of rainfall with a probability of being equaled or exceeded one or more times in a year can be done using the approach below.

1. The length of the rainfall data series is determined (for example n years).
2. Data in each year is broken down from large to small.
3. In each year, the data is taken (k + 1) largest data, where k is the number of events equaled or exceeded in the desired year. So that during n years n x (k + 1) data are obtained.
4. This new data series is sorted from large to small.
5. Rainfall with a probability of being equaled or exceeded k times in a year is data in the order (n x k + 10).

Table 1: Analysis of Design Rain Analysis Results

Return Period (T)	Design Rainfall Frequency Analysis (mm)			
	Normal	Log Normal	Log Pearson Type III	Gumbel
5	188.601	139.711	237.467	193.508
10	205.512	153.019	274.053	221.775
25	221.655	166.901	331.270	257.487
50	235.107	179.428	382.308	283.980
100	245.869	190.123	441.209	310.281

Source: Analysis results, 2024

Based on the results of the rainfall frequency analysis, the ones that meet the design rainfall are based on Log Normal distribution with return periods of 5, 10, 25, 50 and 100 years respectively.

Table 2: Design Rainfall

Return Period (T)	YT	R_{24}
5	2.145	139.711

10	2.185	153.019
25	2.222	166.901
50	2.254	179.428
100	2.279	190.123

By using the Mononobe formula and design rainfall with Log Normal Distribution, the rainfall intensity is obtained as in table 3.

Table 3: Rainfall intensity with various return periods

T (Year)	R_{24} (mm)	I (mm/jam)
5	114	45,0343
10	155	60,9973
25	182	71,8987
50	212	83,5867
100	218	86,1431

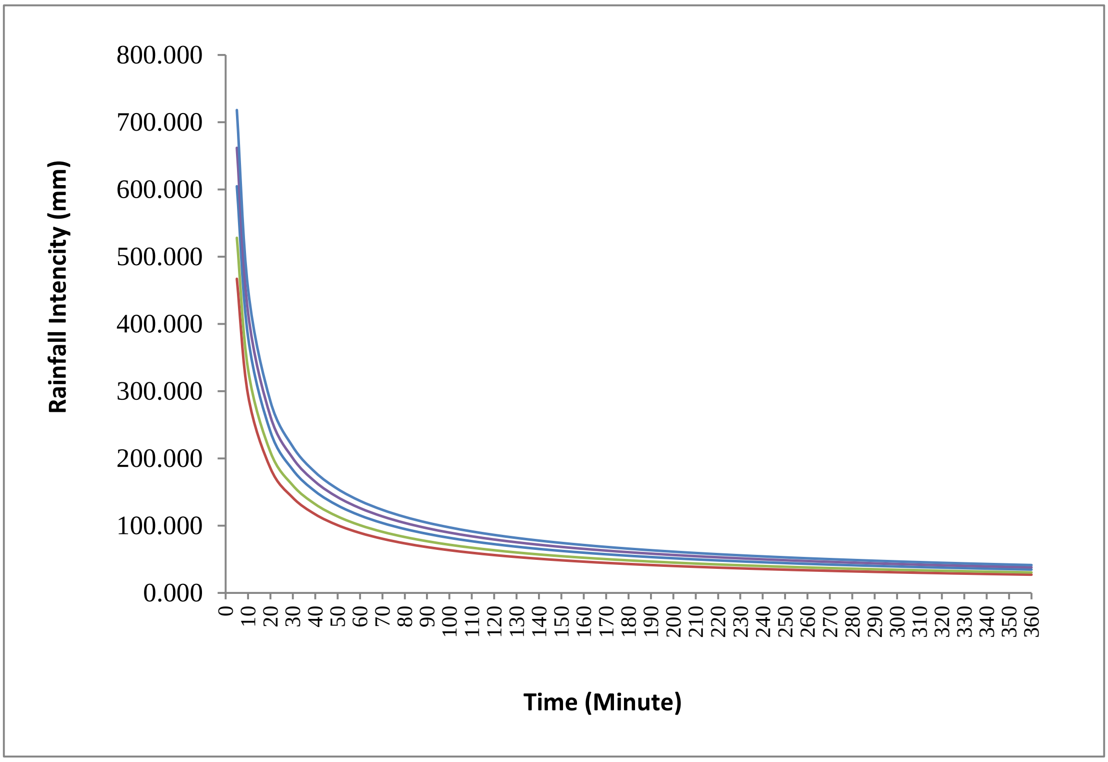

Figure 4: *Intencity Duration Frequency* (IDF) Curve

3.3. Discharge

Q = 0.2778 C I A

with:

Q = Discharge (m^3/sec)

C = Runoff coefficient

I = Rain intensity with duration equal to flood concentration time (mm/hour)

A = Catchment area (km^2)

Q_{10} = 0.2778 . 0.85 . (60.9973 mm/hour) . (2.2341 km^2)

Q_{10} = 14.403 m^3/sec

3.4. Retention Pond Design

The retention pond is designed based on flood discharge (Q_{10}) of 14.403 m^3/sec and inundation area of 2.2341 km^2.

Calculation of Water Volume that must be accommodated with the planned flood discharge value ($Qmax$) = 14.403 m^3/sec, the discharge value of the catchment channel Q = 1.86 $m^3/detik$, concentration time (tc) = 11.45 minutes, and the overflow time value in the channel ($t2$) = 6.40 minutes.
t1 = 1.50 minutes, and t2 = 2.34 minutes.
With this principle, to calculate the volume of water that must be accommodated using the equation:

$\Delta Q = (Qmax - Q) - (ta + tb) / 2\ x\ 60$

So that the volume of water that must be accommodated is obtained (ΔQ) = 37.923 m^3.

3.4. Retention Pond Dimension Analysis

The planned retention pond must be able to accommodate excess water volume and waste carried by the water current. So the volume of the retention pond is 37.923 m^3. The retention pond is planned next to the channel with the height of the pond equal to the height of the channel, which is 1.00 m. With a height of 1.00 m, the retention pond requires a minimum area of 37.92 m^2.

Retention Pond Plan because waste is one of the causes of flooding at the research location, the retention pond is planned to be equipped with a waste filter. Adjusting the area requirements of the retention pond with the existing land conditions, the retention pond plan is planned as follows:

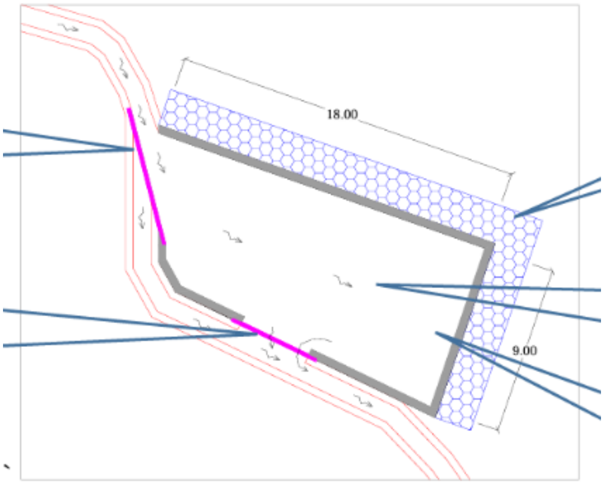

Figure 5. Retention Pond Design

IV. CONCLUSION

This study can be concluded as follows:

1. The planned discharge for the retention pond is 14.403 m^3/sec so it is ideal for the volume of the retention pond as a flood controller in the Sawah village area, Martapura sub-district, East OKU district.
2. The volume of the retention pond (ΔQ) = 37.923 $m3$ with the dimensions of the pond planned next to the channel with a pond height (h) of 1.00 meters, so the retention pond requires a minimum area of 37.92 $m2$.

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